

# GROUND TIRE RUBBER (GTR) IN PAVEMENTS



Increasing the use of recycled content in road & infrastructure workshop

NOVEMBER 17, 2020

- Ground Tire Rubber (GTR) in MassDOT Pavements
- What is Asphalt Rubber”
  - Benefits
- Asphalt Rubber Pavement Performance
  - Asphalt Rubber & Recycled Asphalt Pavement
  - Asphalt Rubber & Warm Mix
- Sample Projects
  - HMA-ARGG
  - Stress Absorbing Membrane Interlayer (SAMI)
- Issues

# Ground Tire Rubber – MassDOT Usage (in Pavements)



- Crack Sealants & joint Sealants (some products)
- Rubber Chip Seal Surface Treatment
- Stress Absorbing Membrane Interlayer (SAMI)
- HMA Pavement Surface (HMA-ARGG)
  - (HMA-ARGG-12.5)
  - (HMA-ARGG-9.5) special provision.
- HMA Open Graded Friction Course
  - HMA-OGFC-AR
  - HMA-Porous Pa
- Ultra-Thin Bonded Overlay
  - UTBO-AR
- Etc....

# Asphalt Rubber

## ■ What is Asphalt Rubber???

- It's a blend of hot liquid asphalt and ground tire rubber.
- 80% Asphalt
- 20% Ground Tire rubber
- First developed in Arizona in the 1960's
- Initially used in Surface Treatments (SAM and SAMI's)
- Now used in HMA Surface Courses – “Thinner Overlays”
- ASTM D-6114 Binder Specification
- Not Proprietary
- Thin Overlays, Open Graded Friction Courses & Pavement Preservation Activities are the biggest market for Ground Tire Rubber in Pavements.



# How is AR made?

## ASTM D6114



### Type II (aka “Wet Process”)

- **15-20% Crumb Rubber**
  - #30-#40 Mesh
  - Processed from Scrap Tires
- **Performance Graded Asphalt**
  - PG58-28 (or)
  - PG64-22
- **Requires On Site blending or at a nearby facility**
- **Reaction process**
  - Elevate Temperature of Liquid Asphalt
  - Mix for 1 hour
    - Rubber particles swell
    - Suspension in Asphalt



# GTR Asphalt Products



Asphalt-Rubber Binder



Terminal Blend Binder

# Why Asphalt Rubber?

## Rubber contains polymers which...

- Raises softening point to above 140° F.
  - Resistant to rutting and shoving
  - Resistant to asphalt migration and drain-down
- Increases low temperature flexibility.
  - Resistance to cracking
- Increases high temperature viscosity.
  - Thicker film coatings on aggregate particles
  - More asphalt = greater resistance to oxidation
  - Increased long term durability
  - Top PG Grading above 80 = Stability















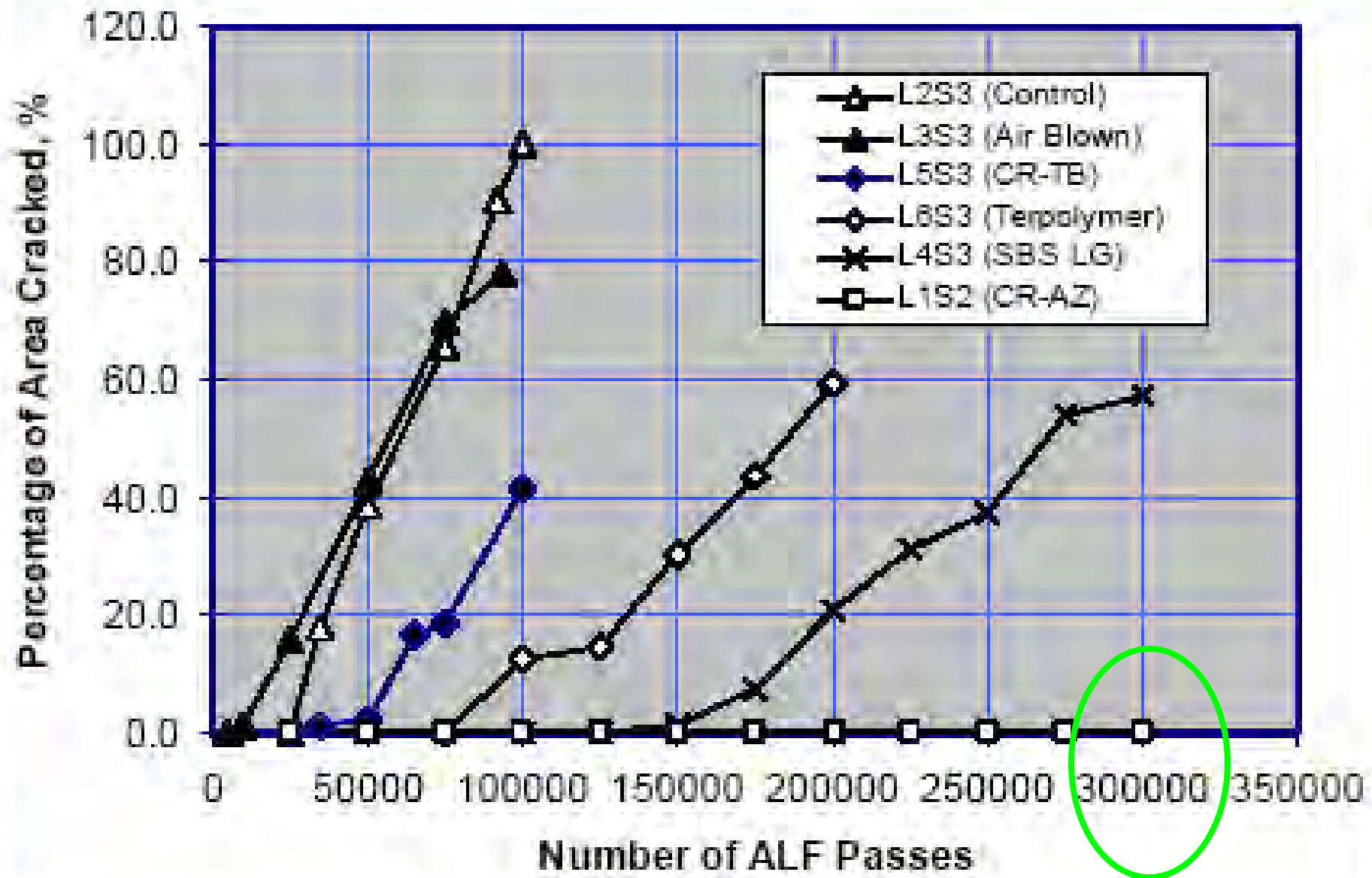
# Why AR Mixes?

## ■ Benefits

- Longer Pavement Life
  - Reduced Rutting
    - AR = Higher Softening Point of PG Binder
  - Reduced Oxidation
    - Thicker Film Coatings = More Binder in Mix
  - Reduced Cracking
    - AR Properties = Greater Flexibility at Cold Temperatures
  - Increased Long Term Durability
  - Reduced Thickness
- Noise Reduction
  - **AR Properties Absorb Tire Noise**
  - Economic Alternative to Sound Barriers
- **“Green” Process**
  - **Reuses Scrap Tires**

# ALF Project Test Sections

|   |   |   |   |   |   |  |   |   |   |   |   |
|---|---|---|---|---|---|--|---|---|---|---|---|
|  |  |  |  |  |  |  |  |  |  |  |  |
| CRMA<br>70-22   | 70-22   | AB  | SBS   | TB<br>CR  | Elvo  | 70-22<br>+<br>Fibers   | 70-22   | SBS<br>64-40  | AB  | SBS   | Elvo  |
| 1   | 2   | 3   | 4   | 5   | 6   | 7  | 8   | 9   | 10  | 11  | 12  |



Percentage of Area Cracked vs. ALF Wheel Load Passes



|         |         |           |         |         |         |
|---------|---------|-----------|---------|---------|---------|
| Lane 1  | Lane 2  | Lane 3    | Lane 4  | Lane 5  | Lane 6  |
| CR-AZ   | Control | Air Blown | SBS LG  | CR-TB   | TP      |
| 300.000 | 100.000 | 100.000   | 300.000 | 100.000 | 200.000 |



# MassDOT GTR

## Background



- SAMI – 1986 Standard Specifications
  - Rubber Chip Seals as wearing courses (SAM)
  - Stress Absorbing Membrane Interlayers (SAMI) to mitigate cracking SAMI.
- 1991 “ISTEA” Rubber Mandates
  - “Generation 1 AR HMA”
    - 1992 Project – Rt 140 Freetown
      - Conventional HMS
    - 1996 – MassDOT Participated in a NIOSH Study
    - 1997 - I-95 Foxboro Southbound
      - Open Graded HMA (permeable pavement)
  - These designs were not gap graded or modified to take advantage of the AR film thickness.

# “Generation 2” GTR

- **Terminal Blend** – typically less than 10% GTR and are produced at an asphalt terminal.
  - 2004 – “Pavement Preservation” Thin Overlays
  - “Terminal Blends” GTR & Polymer (PGAB 76-34)
    - I-91 Bernardston-Greenfield (2005)
    - Rt 146 Uxbridge-Milville (2006)
    - Rt 2 Gardner-Westminster (2006)
    - I-395SB Webster (2006) OGFC
      - GTR clogged plant screens/filters for AC pump
      - Low Binder Control Strip high speed lane – left in place
        - 5% rather than 6.2% Asphalt Content.
        - Despite low asphalt content, it outperformed the “control” section
- **Asphalt Rubber vs Terminal Blend Demonstration Project**
  - I-295 Attleboro-North Attleboro
  - HMA-ARGG and UTBO-AR.

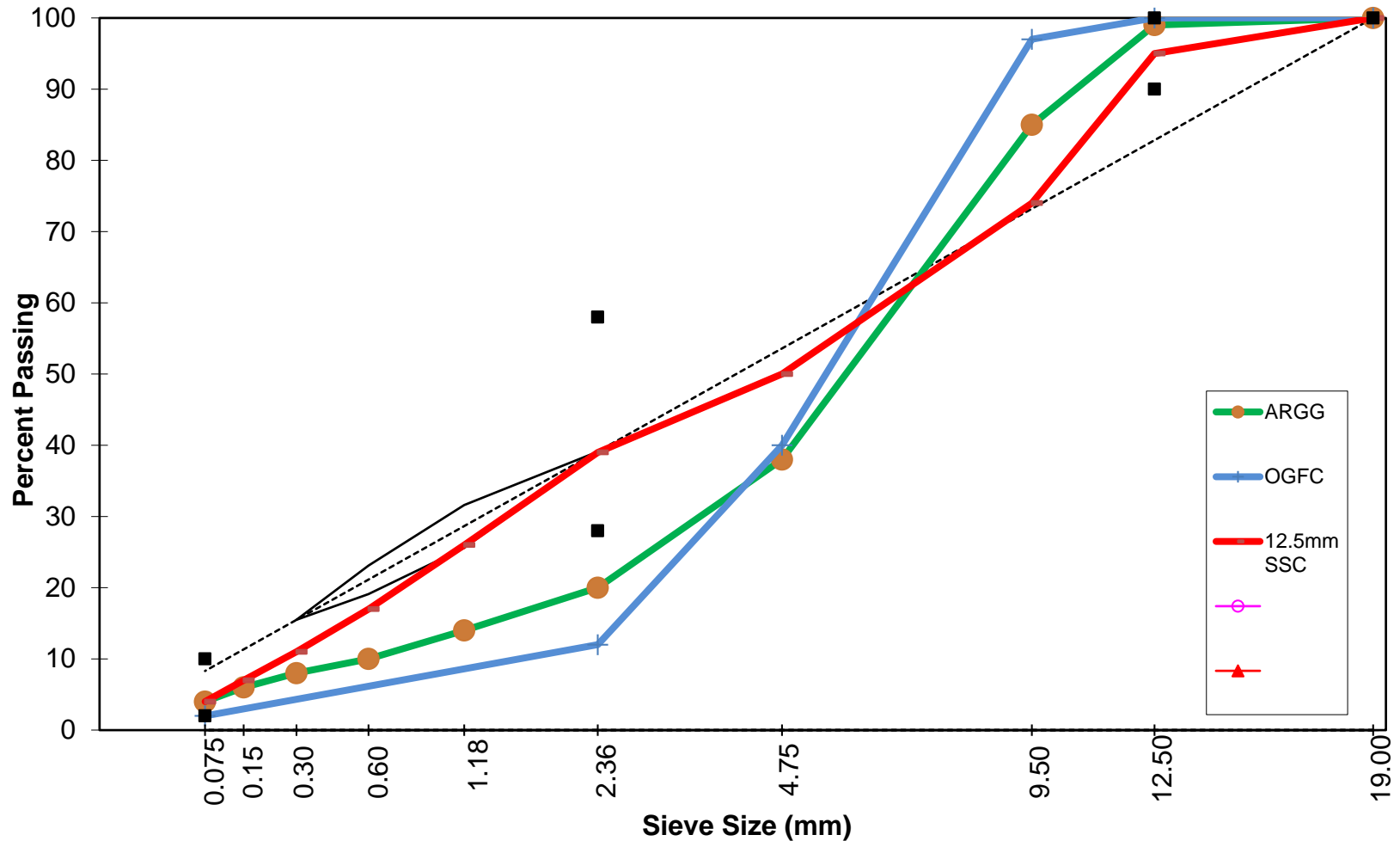
# Asphalt Rubber HMA – “2’nd Generation”



- I-295 Attleboro-North Attleboro Pilot Project
- Terminal Blend - Asphalt Rubber “showdown”.
  - Asphalt Rubber Gap Graded (ARGG) PG 58-28
  - Asphalt Rubber Gap Graded (ARGG-WMA) PG 58-28
  - Bonded Ultrathin Overlay w/PG 64-28
  - Bonded Ultrathin Overlay w/PG 58-28 + AR
- Availability of Terminal Blend GTR Binder
- Bid 2007 - Built 2008
- Construction Changes – Warm Mix .

# Asphalt Rubber-Gap Graded (HMA-ARGG)

### HMA-ARGG vs. OGFC vs. Conventional HMA





# ARGG - Specifications

| <u>Sieve Designation</u> | <u>Percent by Mass Passing</u> | <u>Tolerances</u> |
|--------------------------|--------------------------------|-------------------|
| 19.0 mm (3/4")           | 100                            | ± 0               |
| 12.5 mm (1/2")           | 90-100                         | ± 6               |
| 9.5 mm (3/8")            | 83 - 87                        | ± 6               |
| 4.75 mm (#4)             | 28 - 42                        | ± 6               |
| 2.36 mm (#8)             | 14 - 22                        | ± 4               |
| 1.18 mm (#16)            | -                              | -                 |
| 0.075 mm (#200)          | 0 - 6                          | ± 1               |

## Property

## Criteria

Air Voids

3 - 6 %

Voids in Mineral Aggregates (VMA)

18 - 23 %

Draindown

0.3 % maximum

% Binder content\*

**7.6 % minimum**

PGB Content – Specification limits\*\*

±0.4%

PGB Content – Engineering limits\*\*

±0.6%

# Warm Mix Asphalt & Recycled Asphalt Pavement (RAP)



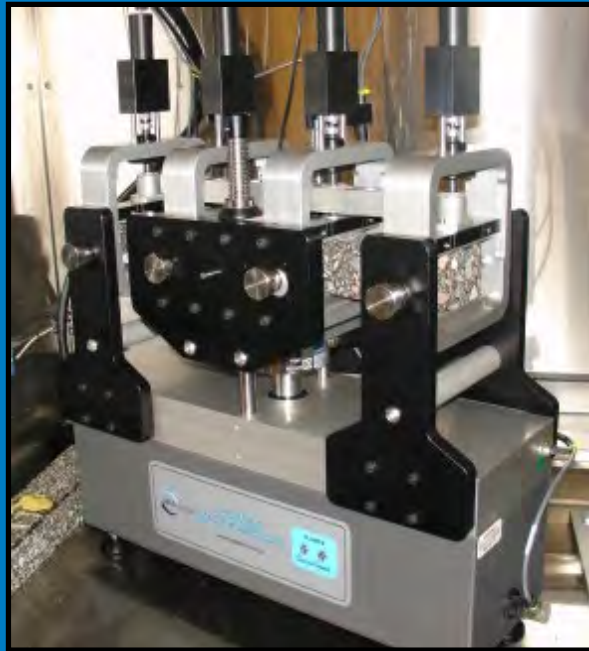
- Warm Mix Asphalt (WMA)
  - Additive which Lowers Production & Compaction Temps
- I-295 Attleboro Demo Project
  - Advera (Zeolite WMA)
- Rt 3 Plymouth Late Season Paving (35 F)
  - Sonnewarm WMA
    - Increased compaction time
    - No impact to stability or moisture damage
    - No temperature reduction attempted.
- **I-495 HAMS – questioned why “no-RAP”???**
  - Performance Questions using WMA & RAP.
  - Task under ISA with UMASS Dartmouth HSRC.

# UMass Dartmouth HSRC Plant Produced Mixture Comparison



- DOT assigned a task to UMASS HSRC for comparison of plant produced ARGG mixture to 12.5mm Superpave and a latex modified control mix.
- Testing included:
  - Beam Fatigue
  - Dynamic Modulus
  - Flow Number
  - Hamburg Wheel Tester
  - Overlay Tester
  - TSRST.

# Fatigue – Four Point Flexural Beam



Testing in Accordance with  
AASHTO T321

- Specimens are fabricated at a target air void level of  $7.0 \pm 1.0\%$
- Testing conducted in strain control mode
- Loading Frequency = 10Hz
- Sinusoidal Wave Form
- Failure Criteria = 50% reduction in initial stiffness per AASHTO T321 method

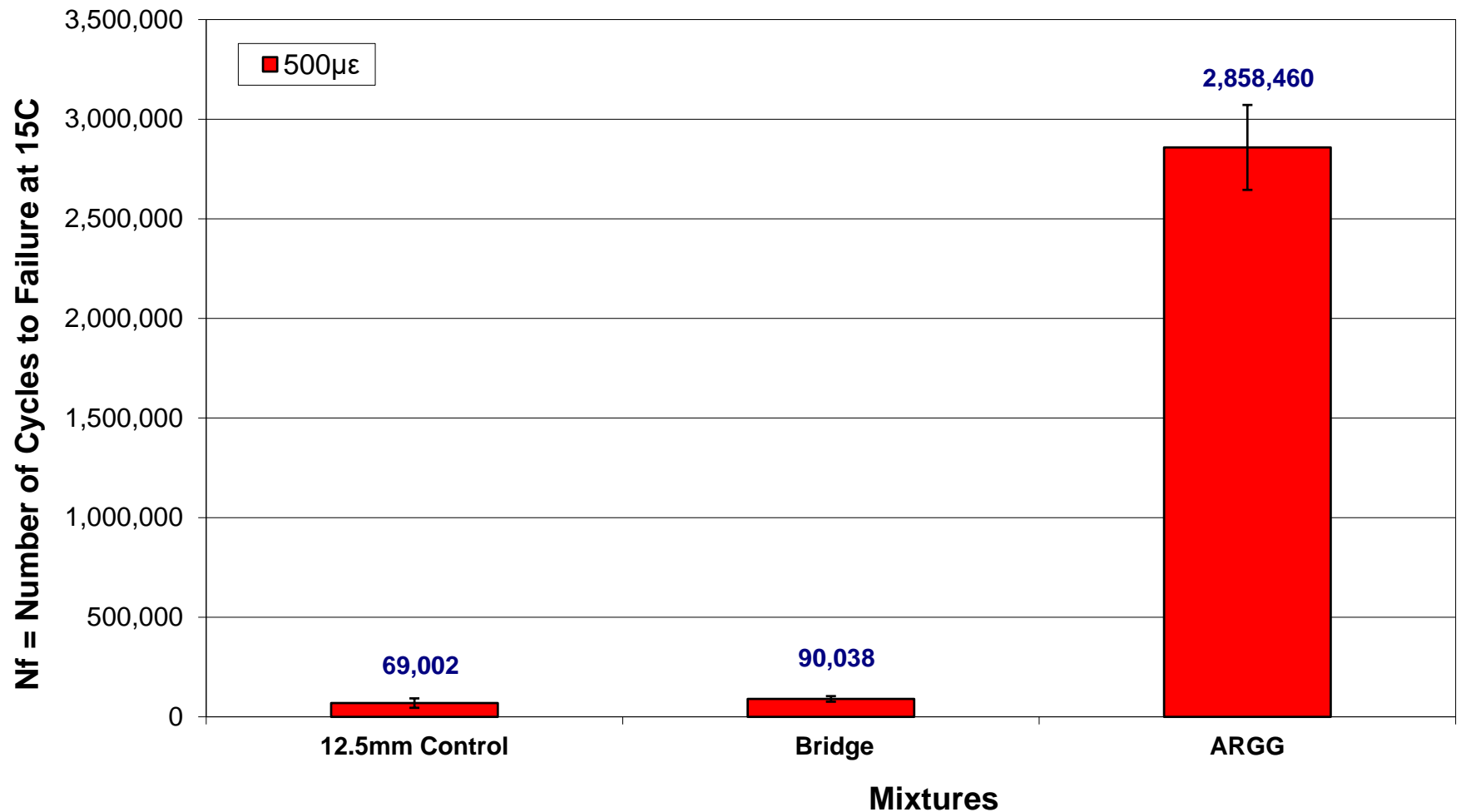
| Temperature | Strain Level  |
|-------------|---------------|
| 15°C (59°F) | $\mu\epsilon$ |



# Plant Mix

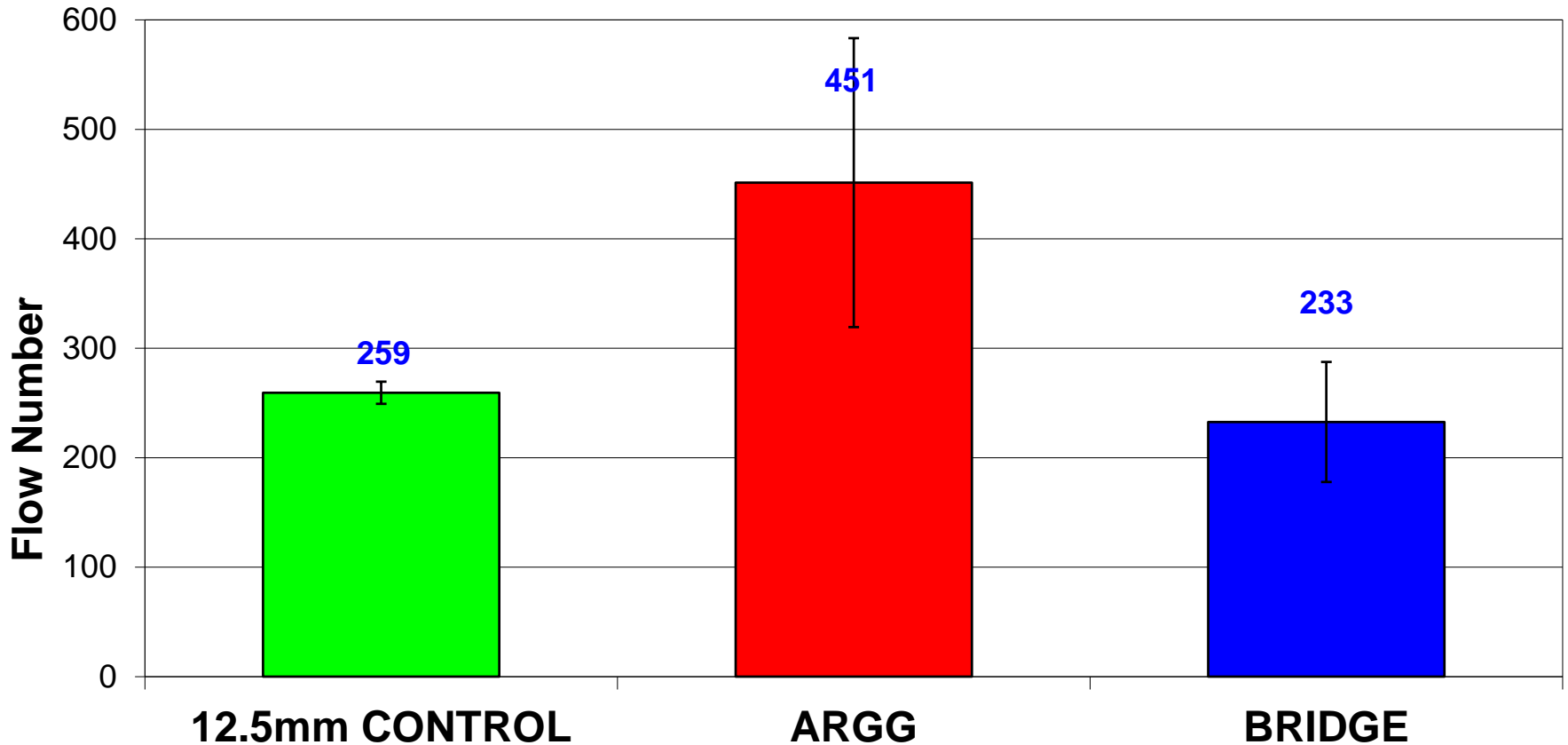
## Beam Fatigue (500 $\mu$ strain)

AASHTO T321 Beam Fatigue Nf to 50% Reduction in Initial Stiffness



# Plant Mix Flow Number

Flow Number - AASHTO TP79 - 50°C 600 kPa Deviator Stress  
MassDOT Control vs ARGG vs BRIDGE



# Rutting / Moisture Susceptibility - HWTD

AASHTO T324: Standard Method of Test for Hamburg Wheel-Track Testing of Compacted Hot Mix Asphalt (HMA)



Water at 45°C (113°F) • Duration of 20,000 passes • SGC specimens at  $7.0 \pm 1.0\%$  air voids

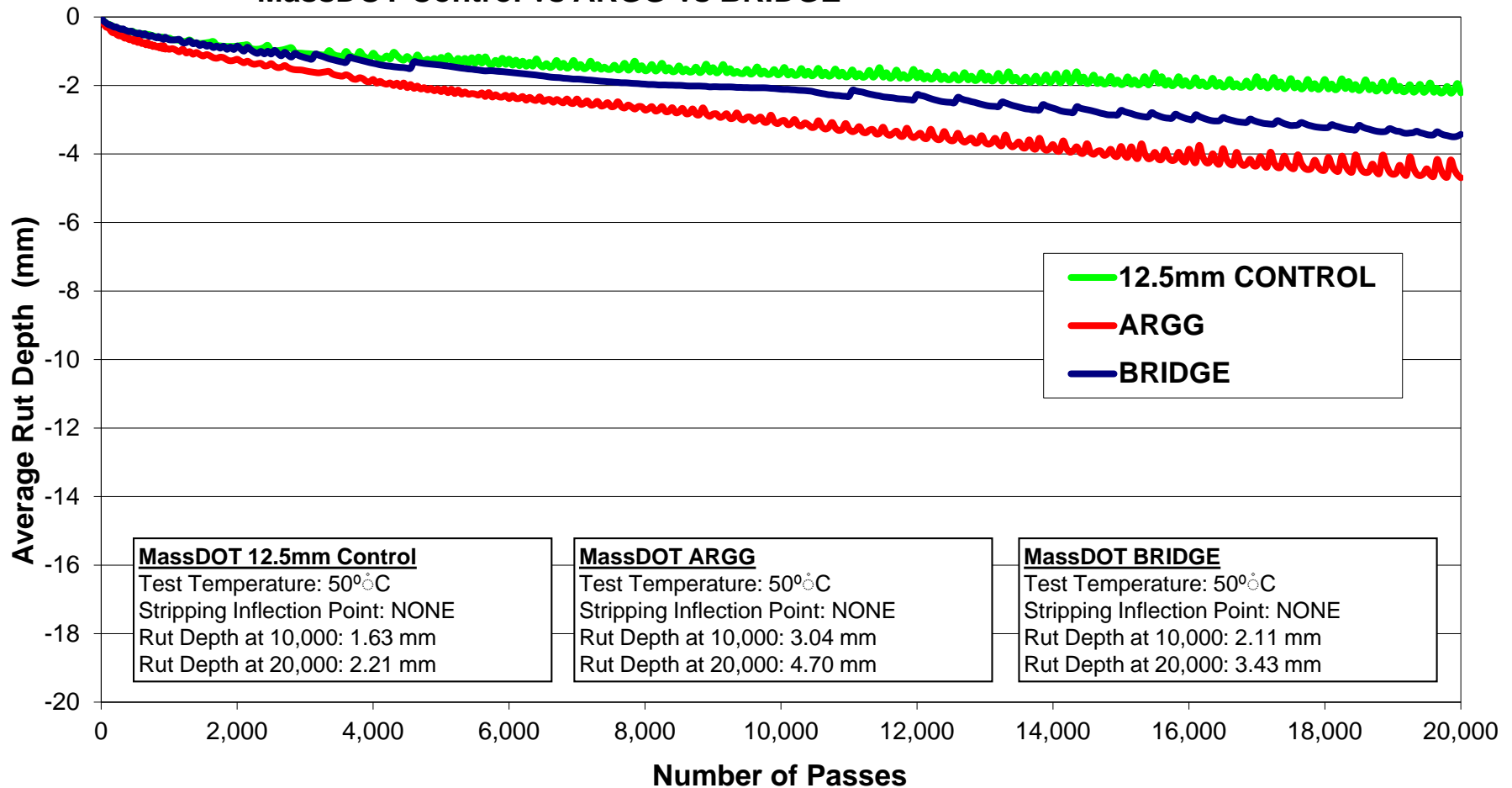
## MassDOT Pass/Fail Criteria

Maximum rut depth of 12.5 mm after 20,000 passes combined with no SIP before 15,000 passes.

# Plant Mix

## Hamburg Wheel Testing

**AASHTO T324 Hamburg Results**  
**MassDOT Control vs ARGG vs BRIDGE**

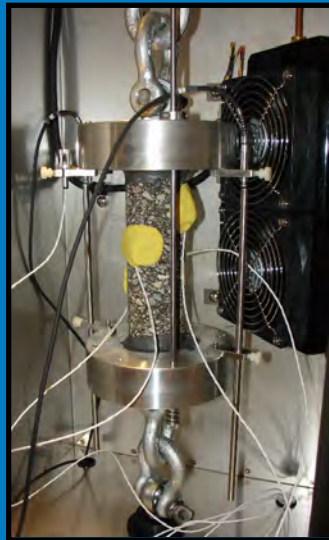


**MassDOT 12.5mm Control**  
 Test Temperature: 50°C  
 Stripping Inflection Point: NONE  
 Rut Depth at 10,000: 1.63 mm  
 Rut Depth at 20,000: 2.21 mm

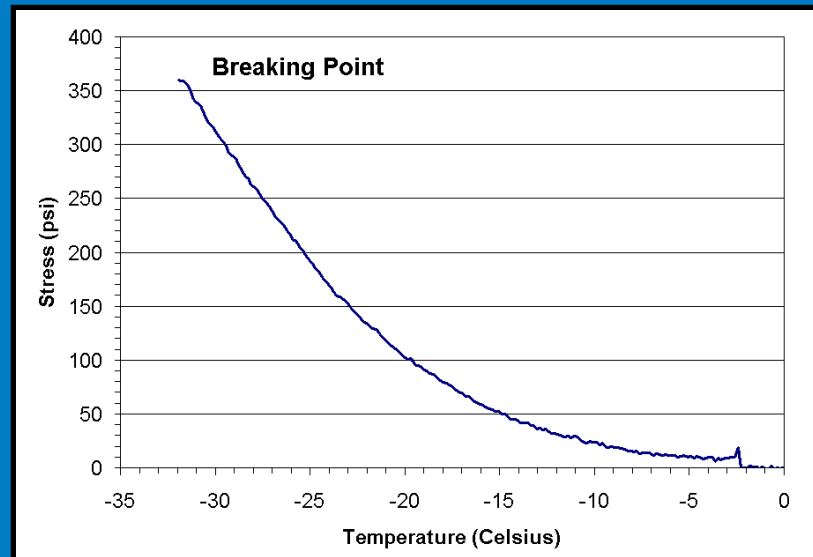
**MassDOT ARGG**  
 Test Temperature: 50°C  
 Stripping Inflection Point: NONE  
 Rut Depth at 10,000: 3.04 mm  
 Rut Depth at 20,000: 4.70 mm

**MassDOT BRIDGE**  
 Test Temperature: 50°C  
 Stripping Inflection Point: NONE  
 Rut Depth at 10,000: 2.11 mm  
 Rut Depth at 20,000: 3.43 mm

# Thermal Cracking Test – TSRST



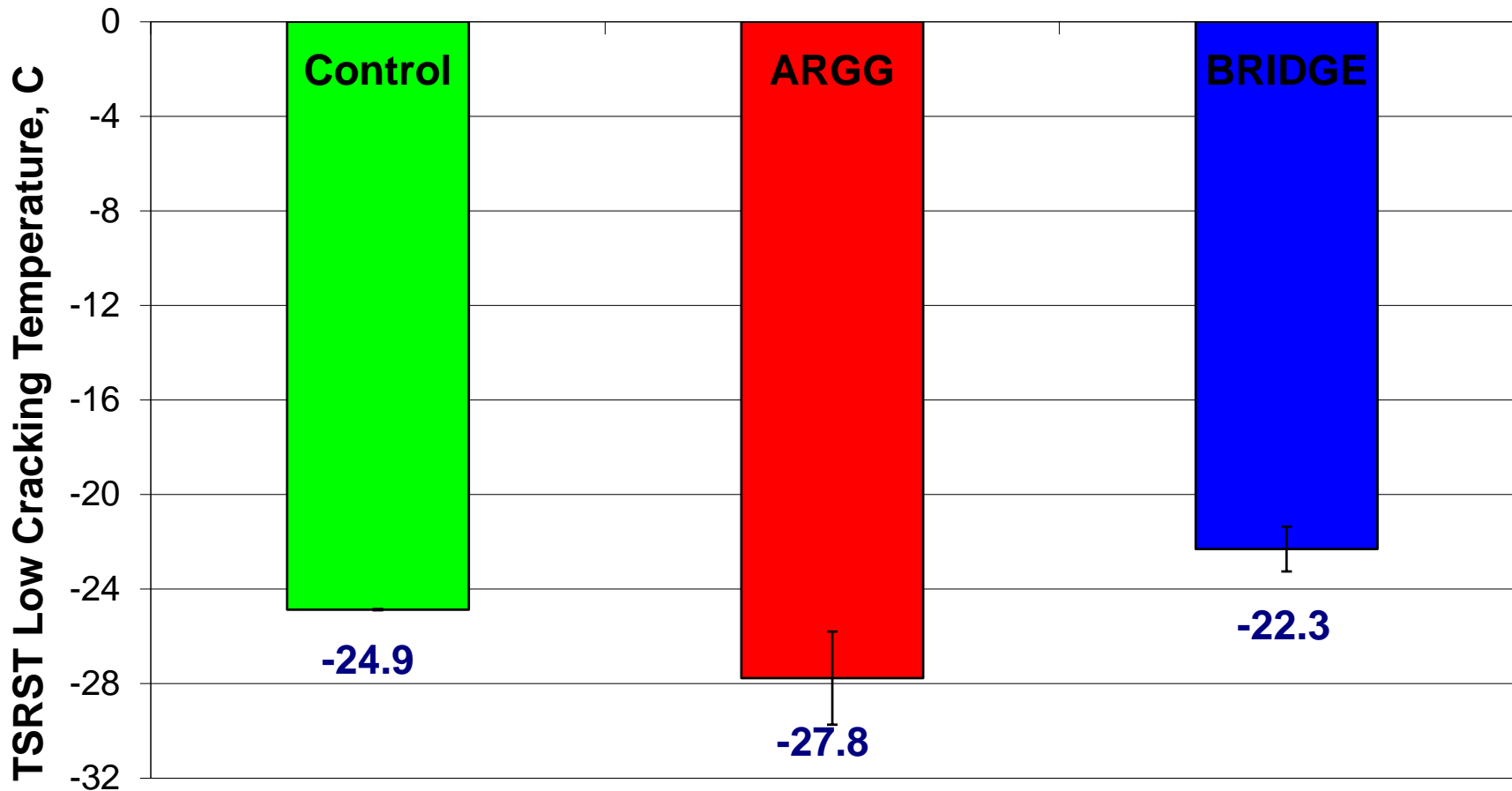
- Superpave gyratory specimens utilized.
- Cooling Rate of  $-10^{\circ}\text{C}/\text{hour}$ .
- Testing in accordance with AASHTO TP10-93.





# Plant Mix TSRST Results

**TSRST Results - AASHTO TP10**  
MassDOT Control vs ARGG vs BRIDGE



# Reflective Cracking – +Overlay Tester



- Test Temperature = 15°C (59°F)
- Test Termination at 1,200 cycles or 93% Load reduction
- Testing in accordance with Tex-248-F

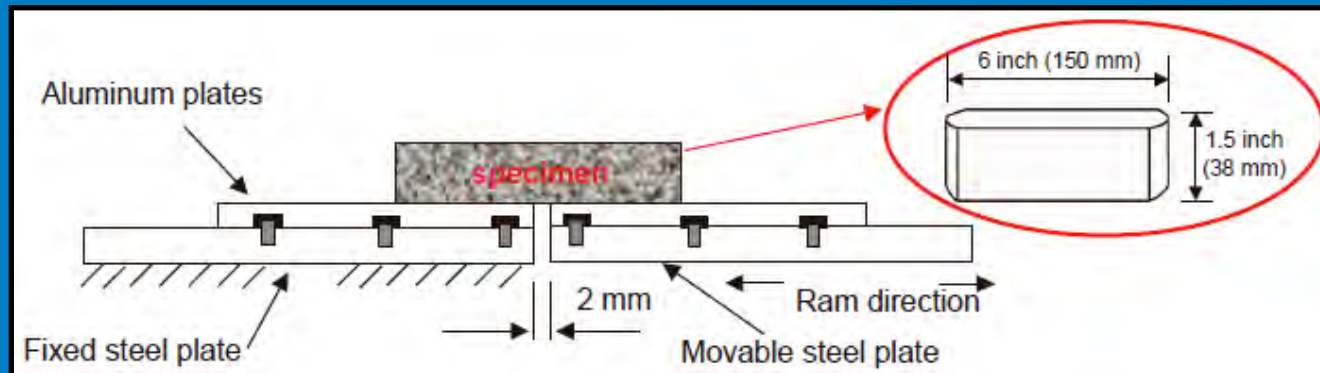
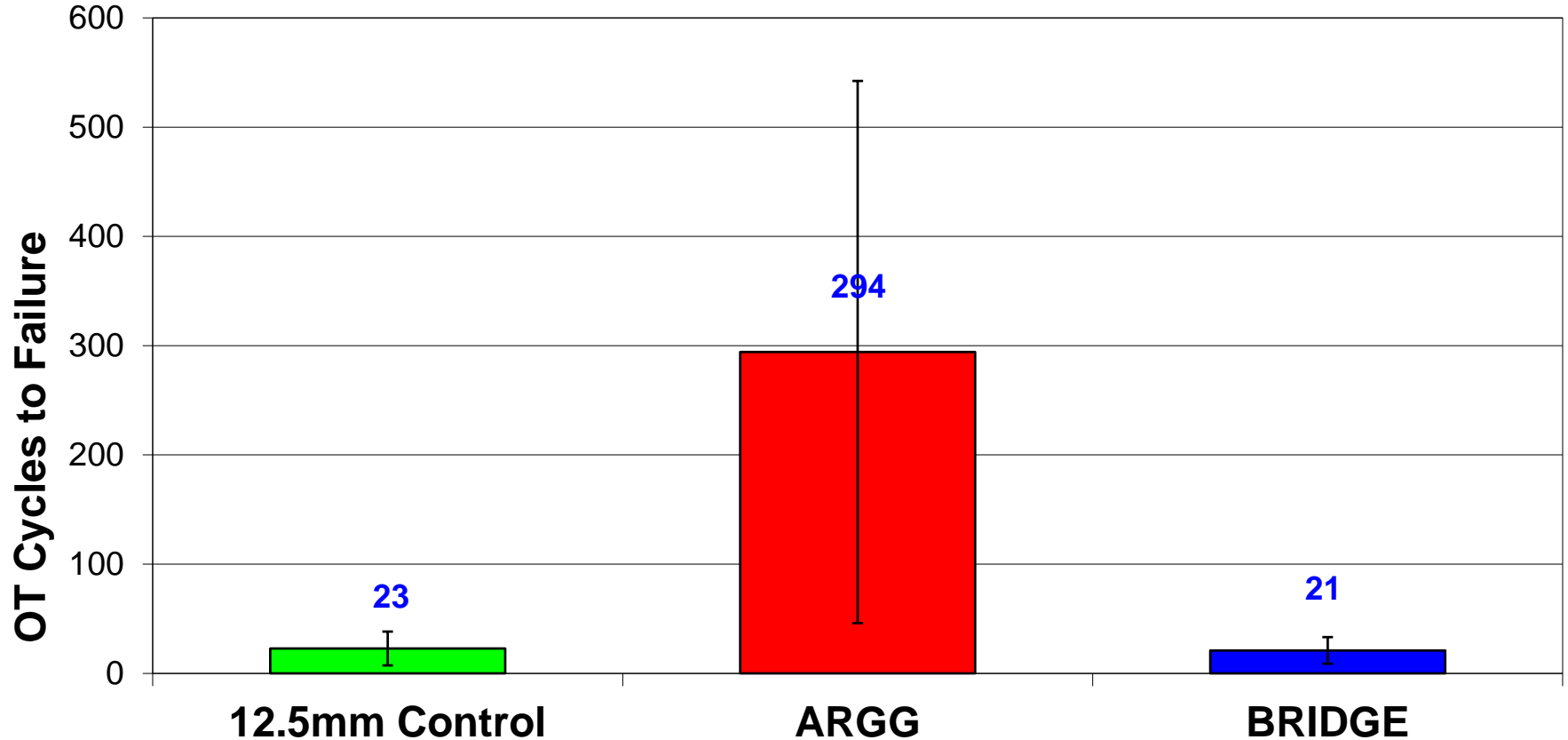


Diagram from: Zhou et al. "Overlay Tester: Simple Performance Test for Fatigue Cracking" Transportation Research Record: Journal of the Transportation Research Board, No. 2001, Transportation Research Board of the National Academies, Washington, D.C., 2007, pp. 1–8.

# Plant Mix Overlay Test Results

Overlay Test Results - Tex-248-F - 15°C  
MassDOT Control vs. ARGG vs BRIDGE

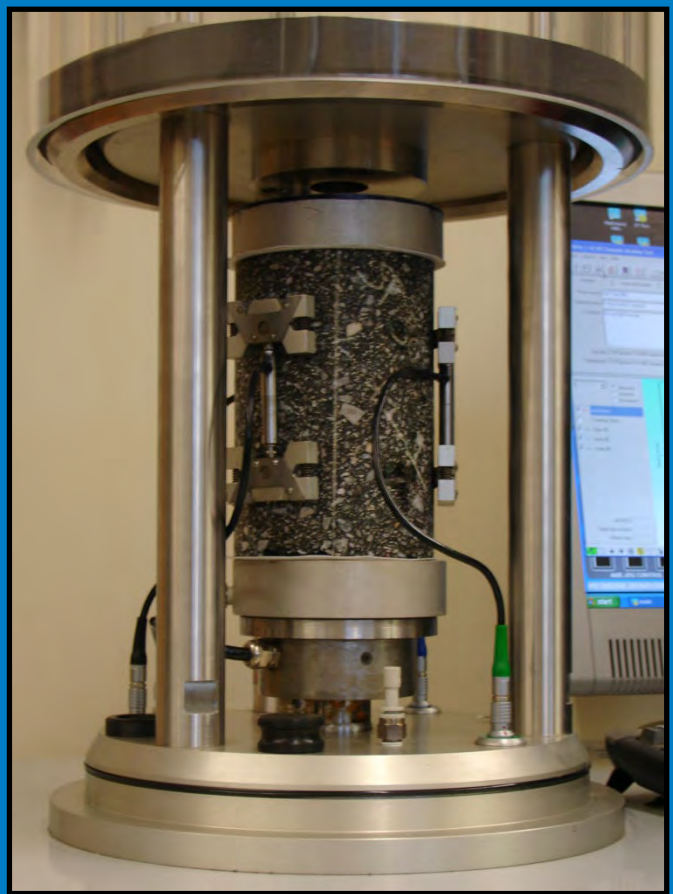


# RAP & Warm Mix Asphalt AR Mixtures

- UMASS Highway Sustainability Research Center undertook an extensive Research Project evaluating use of RAP & WMA with AR.
- WMA - Lower production/placement temperatures, reduced emissions and odors, decreased energy consumption for production & improved environmental working conditions
- Higher binder content for ARGG mixtures may improve mixture cracking resistance, improve rutting performance, and resist aging/oxidation
- Meet the DOT/ industry goal of producing a sustainable, cost effective, and environmentally friendly mixture

# Mixture Stiffness – Dynamic Modulus

Conducted to determine changes in mixture stiffness due to the incorporation of RAP and WMA Technology.



| Temperature | Frequency                 |
|-------------|---------------------------|
| 4°C         | 10 Hz, 1Hz, 0.1Hz         |
| 20°C        | 10 Hz, 1Hz, 0.1Hz         |
| 40°C        | 10 Hz, 1Hz, 0.1Hz, 0.01Hz |

Specimens were fabricated at a target air void level of  $7.0 \pm 1.0\%$ .

AASHTO TP62 in Asphalt Mixture Performance Tester (AMPT)



# Mixture Stiffness

## Conclusions

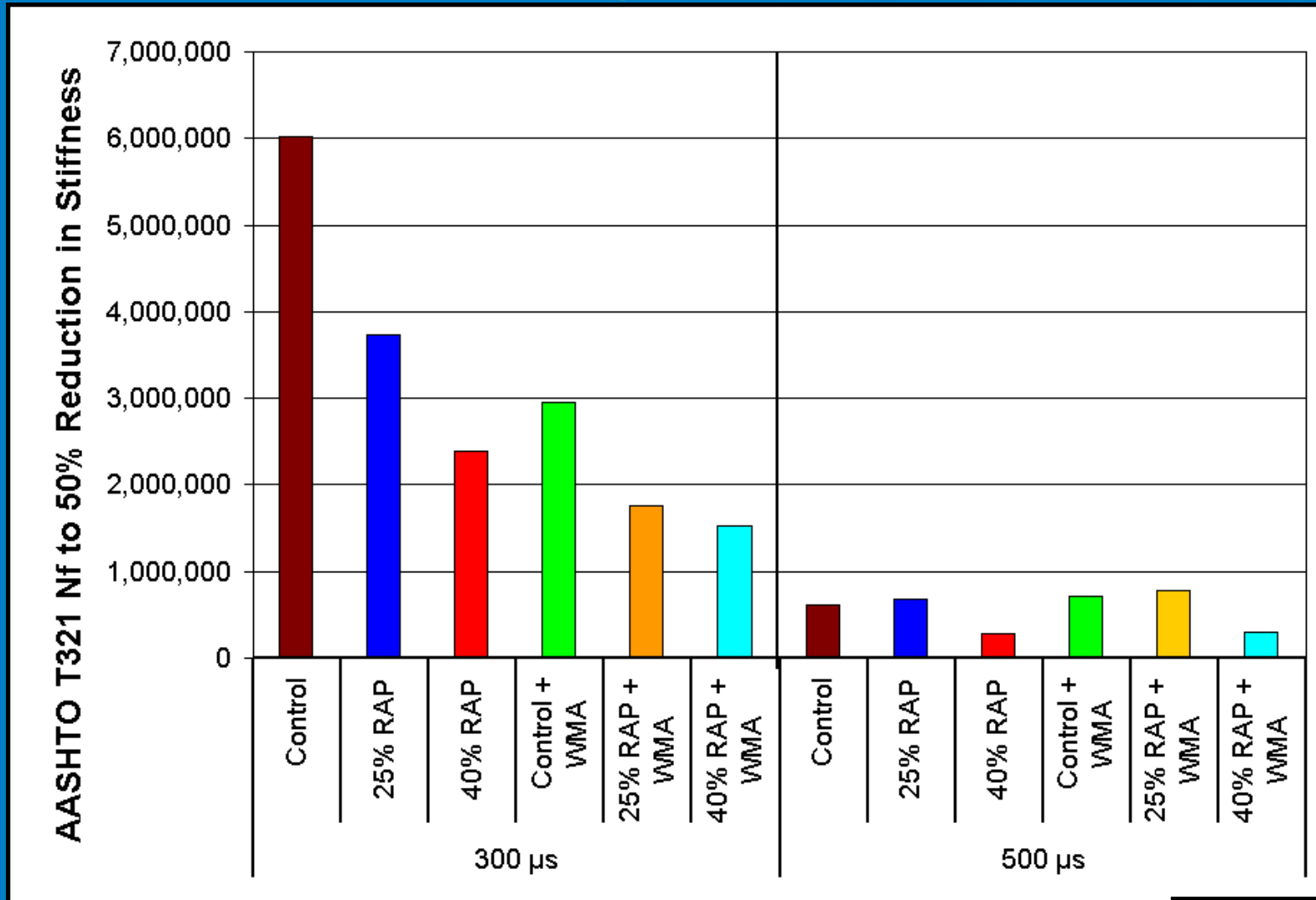
- The addition of RAP to the control mixture resulted in an increase mixture stiffness.
- The stiffness increase in the mixtures containing RAP was mitigated through the use of a WMA technology and corresponding reduced aging temperatures.
- The addition of the WMA technology to the control mixture had little to no effect on the stiffness of the mixture.

# Fatigue –

# Four Point Bending Beam

|                                | Number of Cycles to 50% Initial Stiffness, $N_f$ |                            |                            |
|--------------------------------|--|----------------------------|----------------------------|
| Strain Level,<br>$\mu\epsilon$ | Control  | Control +<br>25% RAP       | Control +<br>40% RAP       |
| 300                            | 6,025,590  | 3,724,655                  | 2,390,822                  |
| 500                            | 614,053  | 677,983                    | 289,898                    |
| 700                            | 544,687  | 197,625                    | 46,895                     |
| 900                            | 25,567   | 24,984                     | 16,255                     |
|                                | Number of Cycles to 50% Initial Stiffness, $N_f$ |                            |                            |
| Strain Level,<br>$\mu\epsilon$ | Control + WMA                                    | Control + 25%<br>RAP + WMA | Control + 40%<br>RAP + WMA |
| 300                            | 2,946,065  | 1,759,123                  | 1,526,473                  |
| 500                            | 705,290  | 775,690                    | 306,746                    |
| 700                            | 196,372  | 99,901                     | 51,134                     |
| 900                            | 21,616   | 27,026                     | 4,697                      |

# Fatigue – Four Point Bending Beam



# Four Point Bending Beam - Conclusions

- The resistance to fatigue cracking decreased with the incorporation of RAP. The same trend was also apparent with the incorporation of the WMA technology.
- At each strain level, the number of cycles to failure for each mixture was reduced when WMA was incorporated.
- For the mixtures incorporating WMA, the mixing and compaction temperatures were dropped 17°C and 13°C respectively. This drop in the temperature might have caused the RAP and AR binders not to comeingle sufficiently.

# Reflective Cracking – Overlay Tester

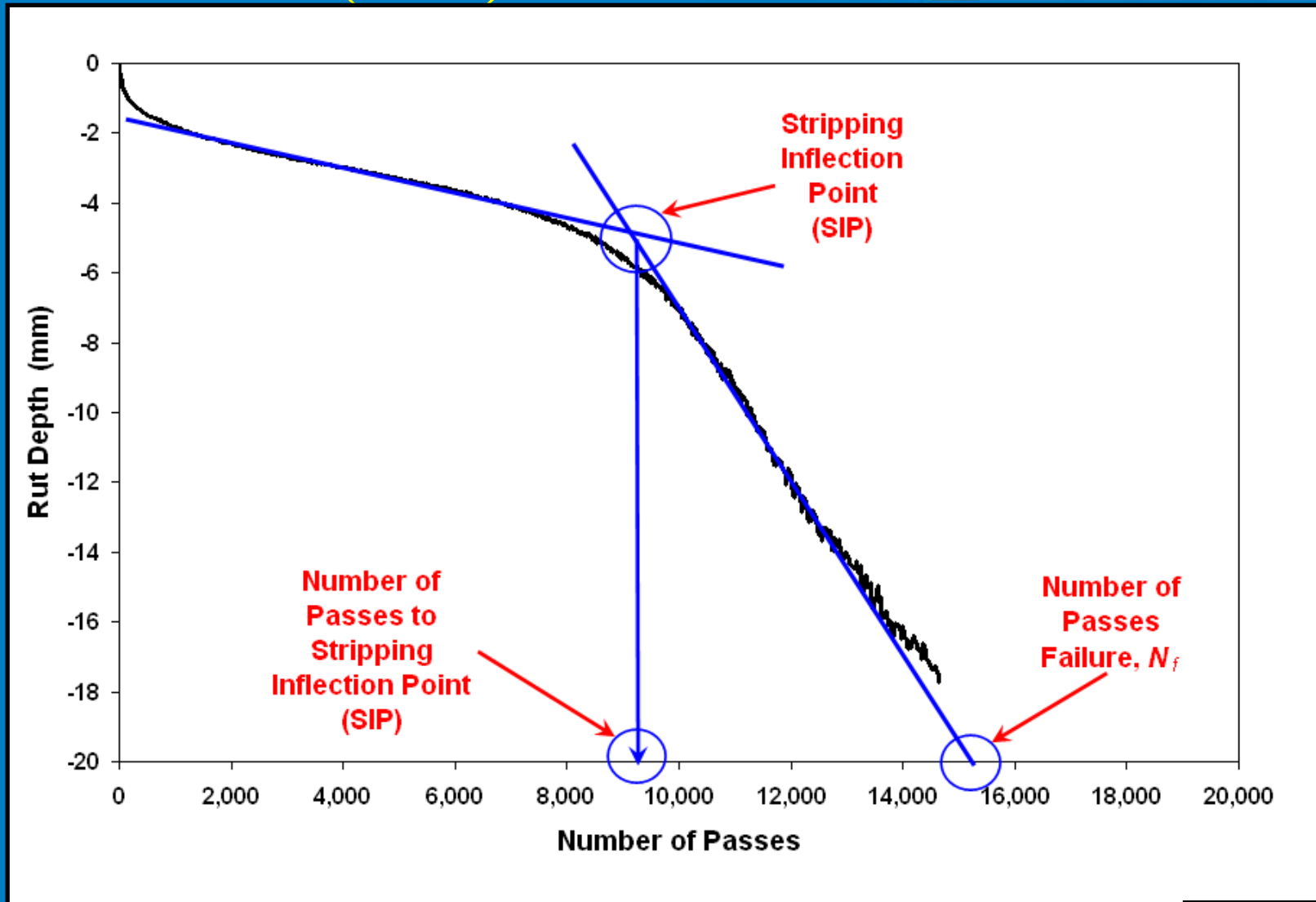
| Mixture          | Average OT Cycles to Failure |
|------------------|------------------------------|
| Control          | 351                          |
| 25% RAP          | 43                           |
| 40% RAP          | 54                           |
| Control + 1% WMA | 275                          |
| 25% RAP + 1% WMA | 64                           |
| 40% RAP + 1% WMA | 21                           |



# Overlay Tester – Conclusions

- The reflective cracking resistance of the mixture decreased with the incorporation of higher amounts of RAP. The same trend was apparent when WMA was incorporated.
- Generally, the OT data agreed with the results of the beam fatigue which showed a reduced cracking resistance for the mixture incorporating WMA.

# Stripping Inflection Point (SIP)



# HWTd Results

| <b>Mixture</b>          | <b>Stripping<br/>Inflection Point</b> | <b>Average Rut<br/>Depth at<br/>10,000<br/>Passes (mm)</b> | <b>Average Rut<br/>Depth at<br/>20,000<br/>Passes (mm)</b> |
|-------------------------|---------------------------------------|--|--|
| <b>Control</b>          | <b>NONE</b>                           | <b>0.88</b>  | <b>1.09</b>  |
| <b>25% RAP</b>          | <b>NONE</b>                           | <b>0.41</b>  | <b>0.51</b>  |
| <b>40% RAP</b>          | <b>NONE</b>                           | <b>0.23</b>  | <b>0.28</b>  |
| <b>Control + 1% WMA</b> | <b>NONE</b>                           | <b>0.45</b>  | <b>0.65</b>  |
| <b>25% RAP + 1% WMA</b> | <b>NONE</b>                           | <b>0.14</b>  | <b>0.23</b>  |
| <b>40% RAP + 1% WMA</b> | <b>NONE</b>                           | <b>0.85</b>  | <b>0.96</b>  |

NONE = Mixture passed 20,000 cycle test with no SIP.

# HWTD Conclusions

- All mixtures evaluated passed the moisture susceptibility testing in the HWTD.
- The magnitude of the average total rut depth observed at the end of each test was less than 1.10 mm (0.043 inch).

# Workability Evaluation

- Mixture workability evaluation was conducted to determine the impact of RAP, AR and/or WMA on mixture workability.
- Workability evaluation was conducted using prototype device designed and built by UMass Dartmouth known as the Asphalt Workability Device (AWD).
- The AWD operates on the torque measurement principles.



# Workability Evaluation

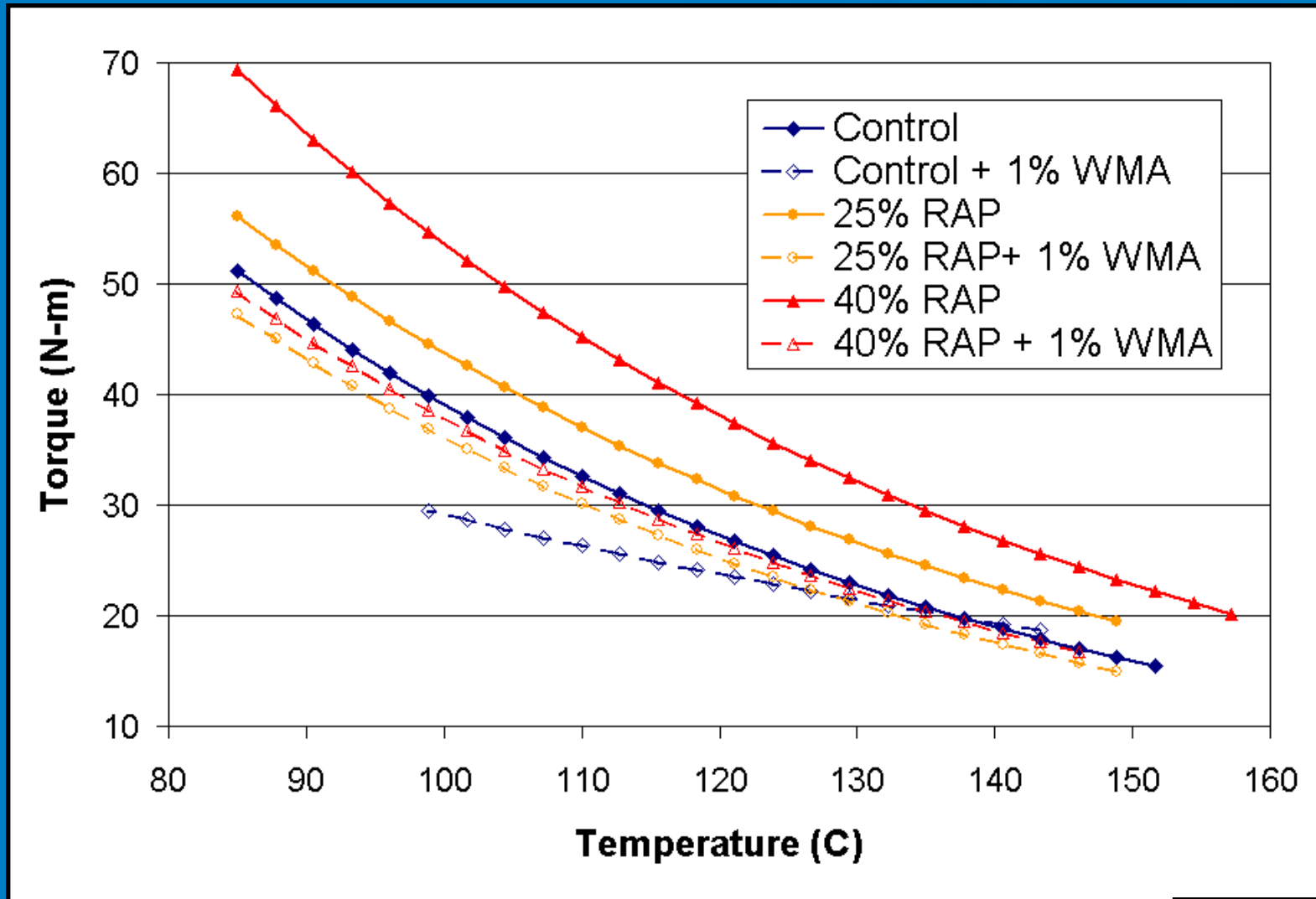


UMass Dartmouth AWD

AWD Paddle  
Configuration



# Workability Results



# Workability Conclusions

- Mixtures without the WMA technology showed that as the amount of RAP incorporated into the mixture was increased there was a corresponding decrease in mixture workability (i.e. increase in torque).
- Overall, the addition of the WMA improved the workability of the mixtures with RAP to a level similar to the control mixture without RAP and WMA.

# Implementation of RAP & WMA in AR Mixes..



- How were any results from the Study Implemented by the DOT?
  - WMA required in all Asphalt Rubber Mixtures.
  - 10% RAP Permitted in ARGG!
  - Must be capable of lowering production temperatures to 280F.
  - DOT has waived its initial temperature requirement of 55F for placement of ARGG.



# Hot Mix Asphalt- ARGGG





# Warm Mix Asphalt– ARGG





## As a result of this research..

- Specified ARGG as an overlay on Composite (HMA over Jointed PCC Roadways.
- Specified ARGG on some of the most critical roadways requiring high levels of pavement performance.
- OGFC-AR used on dozens of miles of Interstate Highways.
- Full-Depth Porous Pavement containing AR and shingles for highway median.
- Millions of tons of Asphalt Rubber mixtures placed statewide.

# Route 8 Cheshire Lanesboro Stress Absorbing Membrane Interlayer Demo Project



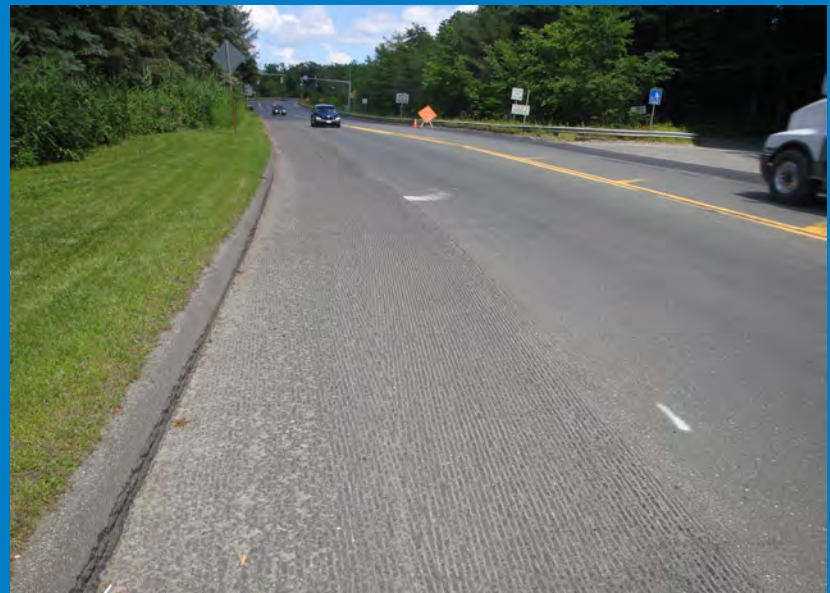
- MassDOT specifies Stress Absorbing Membrane Interlayers (SAMI) to mitigate reflective cracking in some applications. Item #466.
- SAMI can be placed independent of an overlay and left open to traffic.
- Four test sections were constructed on Route 8 in the towns of Cheshire- Lanesboro.
- Two Sections included a Rubber Chip Seal SAMI.
  - SAMI & HMA Overlay
  - SAMI & Bonded Thin Overlay

# Route 8 Cheshire Lanesboro Construction





# Route 8 Cheshire Lanesboro Construction





# Cheshire-Lanesboro – Two Years Later HMA Overlay on Shoulder – No SAMI





# Cheshire Lanesboro – Two Years Later No SAMI - Core





# Cheshire Lanesboro HMA over SAMI





## Cheshire - Lanesboro HMA over Rubber Chip Seal SAMI

- First Core on shoulder – no SAMI
- Second Core through SAMI
- Effective on most longitudinal cracking
- Effective on less light to moderate transverse cracking





# Cheshire Lanesboro HMA over Rubber Chip Seal SAMI







## Route 8 Cheshire Lanesboro

- HMA over Rubber Chip Seal SAMI
- Crack stops at SAMI.
- Effective on most longitudinal cracking.
- Effective on less severe transverse cracking.

# Cheshire Lanesboro Bonded Thin Overlay on Rubber Chip SAMI





# Kernwood Drawbridge Salem, MA





# Kernwood Drawbridge Salem, MA





# I-495N Milford – Southborough

OPER ROUTE Rn Com D L W  
 93M489 0495N 01 1 1 1  
 59.865 40.6 08/10/21



OPER ROUTE Rn Com D L W  
 JHS42P 0495N 01 1 1 1  
 59.876 42.3 09/10/23



## Ride Statistics

| ROUTE | FROM  | TO    | LIRI  | RIRI  | AVG IRI | COMMENTS | COLLECTION YEAR | PROJECT # |
|-------|-------|-------|-------|-------|---------|----------|-----------------|-----------|
| 0495N | 50.55 | 61.67 | 83.94 | 81.17 | 82.55   | Before   | 2008            | 54488     |
| 0495N | 50.55 | 61.67 | 37.89 | 52.86 | 45.37   | After    | 2009            | 54488     |

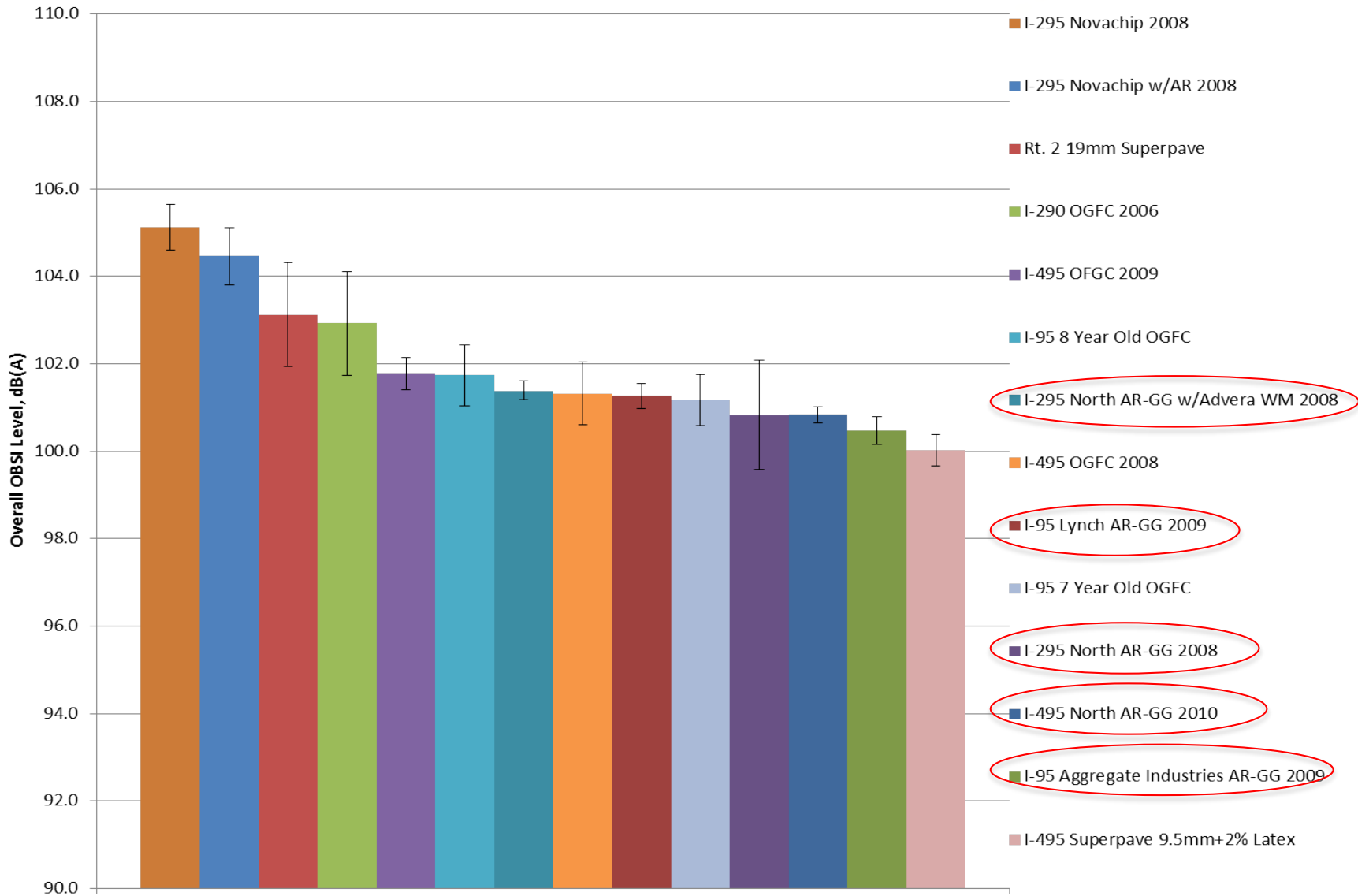
# I-495N Milford–Southborough “10 Years Later”



## Reduction In IRI After Project Completion

| ROUTE | FROM  | TO    | LIRI  | % REDUCED | RIRI  | % REDUCED | AVG IRI | % REDUCED |
|-------|-------|-------|-------|-----------|-------|-----------|---------|-----------|
| 0495N | 50.55 | 61.67 | 46.05 | 54.9%     | 28.31 | 34.9%     | 37.18   | 45.0%     |

## Overview of Tested Materials in MA



# Which Highways have projects using Ground Tire Rubber?

- Rt 1, Rt 2, Rt 3, Rt 8, Rt 9, Rt 23, Rt 24, Rt 32, Rt 44, Rt 68, Rt 128, Rt 140, Rt 213, etc...
- I-84, I-90, I-91, I-93, I-95, I-190, I-195, I-290, I-291, I-295, I-391, I-395, I-495



# Issues?

- Yes, but fortunately not related to performance.
- Even with Warm Mix, AR mixtures require higher production temperatures than unmodified mixtures.
  - Modified asphalts have an odor (GTR, SBS, SBR, etc..).
  - Elevated production temperatures have an odor.
  - HMA Plants need to work with DEP and local officials to address any issues (not just AR).
  - MassDOT has bid AR mixes using a SSC-HP Specification having performance targets.
- Mobilization of AR blending equipment and connecting to an HMA plant has an added cost.
- AR mixtures are more costly than unmodified mixtures.



# Summary

- **Hopefully, this presentation has conveyed the pavement research and testing we perform prior to using new technology.**
  - **Dr. Walaa Mogawer at the UMass Dartmouth Highway Sustainability Resource Center (HSRC) has provided years of testing, expertise and support of these efforts.**
- **GTR recycled into roadways because it improves pavement performance.**
  - **Tire disposal considered ancillary benefit.**
  - **Increased costs of AR pavement offset by improved performance.**
- **MassDOT is implementing specifications which establish minimum standards for many of the performance tests shown on these slides.**

# Thank You!!

# Questions??

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